2010 Highway Capacity Manual

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2010 HCM Format / Schedule

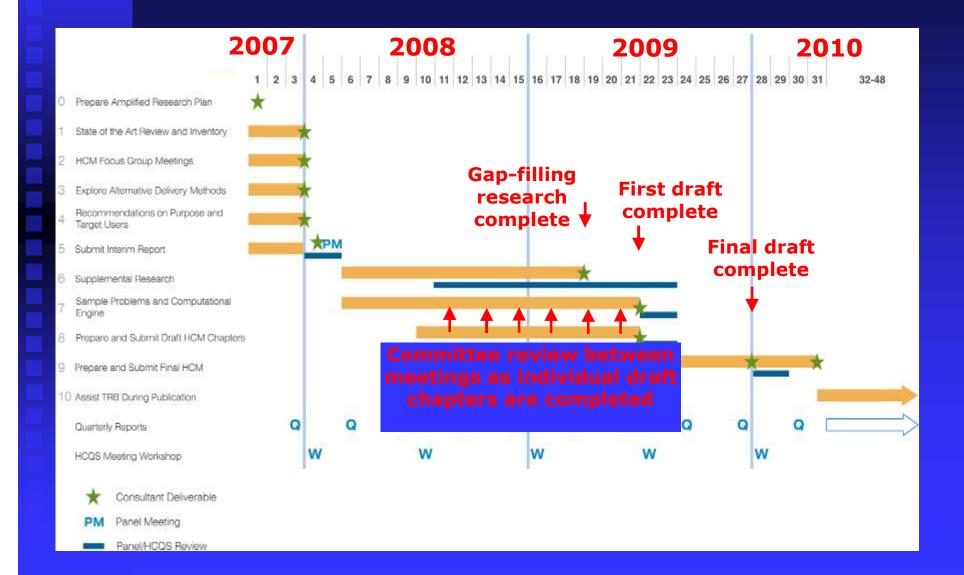
2010 HCM Format 2010 Highway Capacity Manual 5th Edition HCQS Publicatio Research TRB Contact Concepts 2. Uninterrupted Flow Facilities 3. Interrupted Flow Facilities 4. Applications Guide **Updates <u>NELL</u>** Chapter 26 (Adopted): Interchange Ramp Terminals **Chapter X (Evaluation Copy):** Local Urban Streets NCHRP Report XXX: Analysis of Freeway Weaving Sections

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NCHRP 3-92 Project Schedule



Incorporation of New Research

- NCHRP 3-60 (Interchange Ramp Terminals)
- NCHRP 3-64 (HCM Applications Guide)
- NCHRP 3-65 (Applying Modern Roundabouts in the United States)
- NCHRP 3-70 (Multi-Modal Arterial Level of Service)
- NCHRP 3-75 (Analysis of Freeway Weaving)
- NCHRP 3-79 (Predicting Travel Speeds for Urban Streets)
- NCHRP 3-82 (Default Values for HCM)
- NCHRP 3-85 (Guidelines for the Use of Alternative Traffic Analysis Tools)
- NCHRP 20-7 (Two-lane Highways)
- TCRP A-15A (Transit Capacity and Quality of Service 2nd Edition)
- NCHRP 3-92
 - Signalized Intersection Methodology (New Delay Method and Structure Changes reflecting Modern Actuated Control)
 - Gap Acceptance for Six Lane Two-Way Stop Controlled Intersections
 - ♦ 75 Mph Speed Flow Curve for Freeways

Roundabouts

2010 HCM - Roundabouts

Roundabouts: Need for improved HCM procedure
Anchor to empirical U.S. performance
Ability to analyze multilane roundabouts
Able to be calibrated to local and/or changing conditions

Roundabouts – Key Findings

Key Operational Findings from NCHRP 572

- U.S. roundabouts currently exhibit lower capacities than observed in other countries
- Capacity clearly sensitive to geometry in the aggregate (number of lanes)
- Secondary effects of geometry on capacity (e.g., lane width, diameter) masked by variations in driver behavior
- Lane-by-lane analysis of multilane roundabouts needed

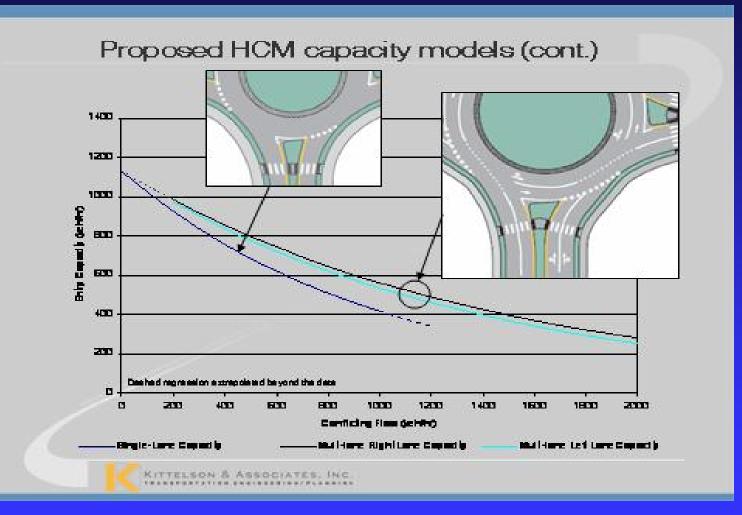
Roundabouts – Procedure

- Can be calibrated to locally measured critical headway and follow-up headway
- Sensitive to turning movement assignment to individual lanes
- Sensitive to lane utilization
- User has ability to assign traffic by lane to match existing or projected conditions
- •Right-turn bypass lanes
 - ◆ -Type 1: Yielding
 - ◆ -Type 2: Non-yielding (acceleration or add lane)
- PCE adjustment for heavy vehicles
- •PHF adjustment for peak 15-minute flows
- LOS definitions

Roundabouts – Alternative Tools

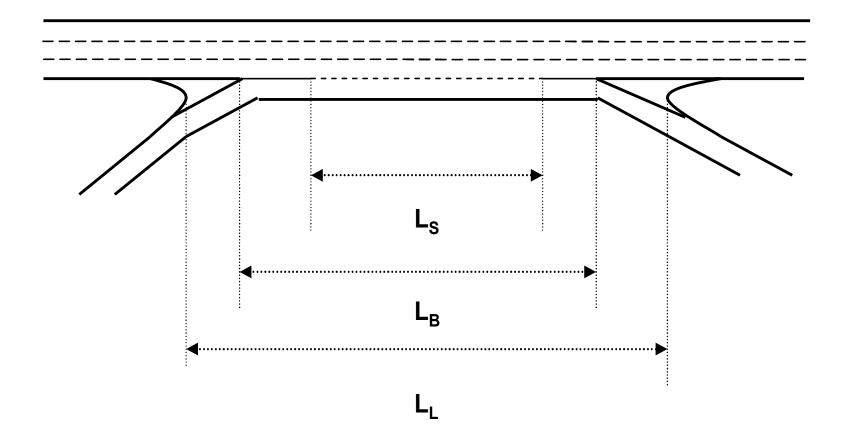
- •2010 HCM will explicitly recognize that HCM procedures are not the only way to analyze problems
- •Applicability of alternative tools to roundabouts
- Geometric configurations not included in model
- Oversaturated conditions requiring multiple-period analysis
- Interaction effects with other intersections
- Overview of characteristics of applicable alternative tools for roundabouts
- –Deterministic tools (e.g., SIDRA, RODEL)
- –Simulation (e.g., VISSIM)
- •Need for calibration

Roundabouts



Weaving

WEAVING -A New Way to Measure Length



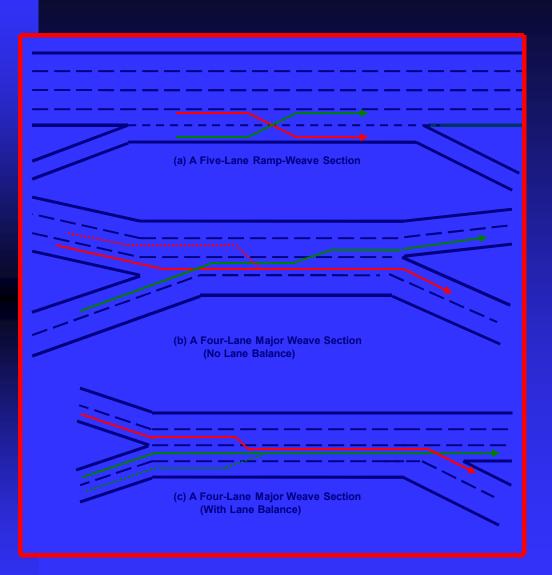
WEAVING - No More:

Configuration Types (A,B, and C) Unconstrained vs. Constrained Operation

New Variables Describing Configuration:

- LC_{MIN} = minimum number of lane changes for all weaving vehicles to successfully complete their desired weaving maneuvers.
- N_{WL} = number of lanes *from which* a weaving maneuver can be made with no more than one lane change.

Two-Sided Weaving Segments Considered as a Special Case



Examples of Configuration Parameters

Determine the Capacity of the Weaving Section

Two Controls on Capacity

Capacity of the weaving section occurs at a point where the average of all vehicles in the section reaches 43 pc/mi/ln.

Capacity of the weaving section occurs at a point where the weaving vehicle demand flow rate reaches:

2,400 pc/h for cases in which N_{wL} = 2 3,500 pc/h for cases in which N_{wL} = 3

If $c_w < v$, Level of service F exists. If $c_w \ge v$, continue to estimate speeds, densities, and LOS.

$$D = rac{v/N}{S}$$

Level of Service Criteria:

LOS	Density (pc/mi/ln)	
	Freeway Weaving Sections	Weaving Sections on C-D Roadways or Multilane Highways
Α	0 – 10	0 – 12
В	>10-20	> 12 - 24
С	> 20 - 28	> 24 - 32
D	> 28 - 35	> 32 - 36
Е	> 35	> 36
F	v/c > 1.00	

Urban Streets

Vision for Chapter 15 Scope



- Urban street segment and facility evaluation
- Auto, bike, and pedestrian modes
- Recognize that sophisticated software tools are available and better suited to some applications

Attributes

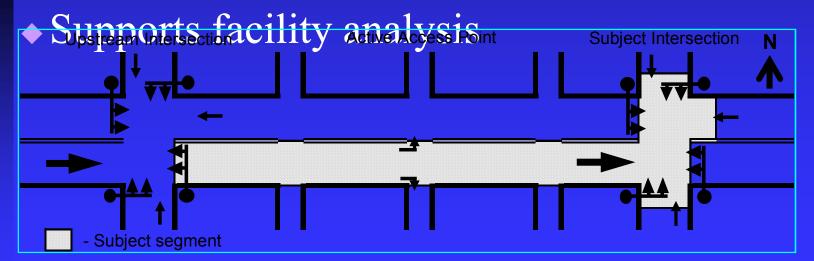
- Useful to planner, designer, traffic engineer
- Pedestrian and Bike Methodology
 - Developed by NCHRP Project 3-70

Automobile Methodology

- Analysis Unit
 - ♦ Segment
 - Includes all movements at signals



Can include movements at access points



Methodology Actuated Phase Duration Prediction Controller operation inputs Simultaneous gap-out Dallas phasing Controller phase inputs Passage time Minimum green Recall Dual entry ◆ HCM Chapter 16 - Appendix B

Methodology Coordinated-Actuated Operation Controller inputs Force mode • Splits \rightarrow used to estimate force off & yield F.O. F.O. Y.P. points Offset & offset reference point Offset

Methodology Arrival Flow Profile Prediction Computes adjusted volume Balance flows between junctions Check and honor capacity constraints to entry flows Computes O-D matrix Upstream Intersection Subject Intersection 11 four $\bullet D^{\cdot}$ ex nod in \diamond \mathbf{R} Chapter 10

Methodology Spillback Prediction Sustained queue backs up for length of segment Model prediction Identify time when spillback occurs ♦ Uses Indicates green needs to be better allocated Indicates simulation should be considered

Performance Measures



Interchange Ramp Terminals

Highway Capacity Manual Chapter Interchange Ramp-Terminals



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Background - Purpose 360:

New Interchange Chapter:
 Develop Accepted Analytic Procedure
 Single and Dual Intersection Interchanges
 SPUI's, Diamonds, Par-Clo's

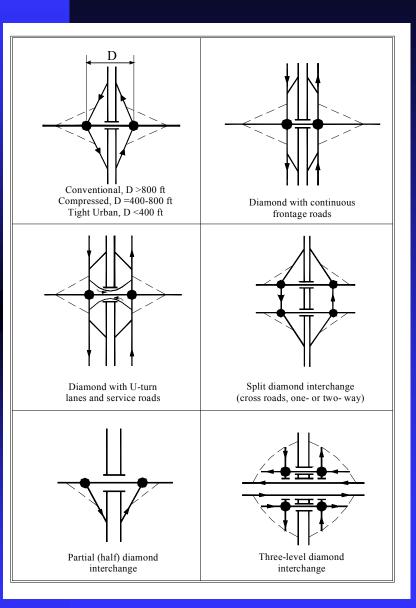
Chapter 26 – Major Changes:

- Identify a Process to Analyze Interchanges
- Level of Service Changed to Reflect Experience
 - Single or Multi-Intersection Device
 - More Complicated than Intersection Analysis
 - More Delay than Typical Intersections
- Origin-Destinations Important
- Operational Impacts of Intersection Spacing
- Lane Utilization (Particularly Left-Turns)
- Demand Starvation (Unused Green)
- Interchange Selection Process

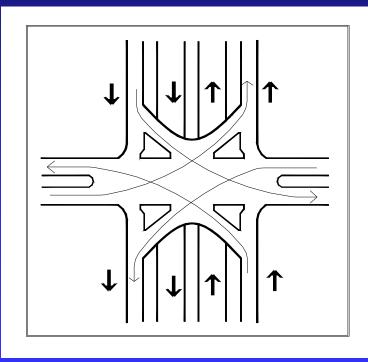
Chapter 26 – Why Needed?

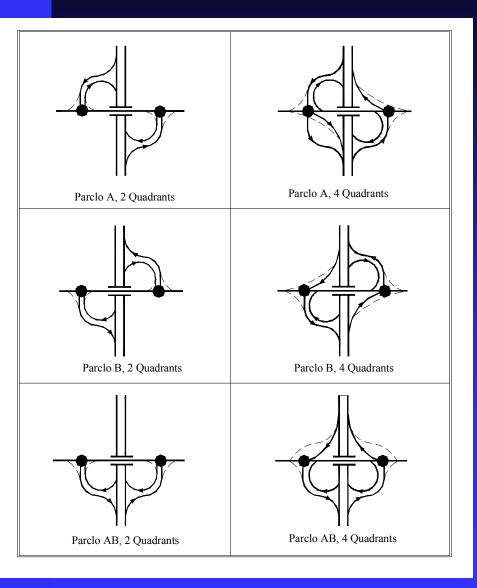
No Guidance Today Pandora's Box of Procedures Used ♦ HCM, Intersections ♦ Simulation ♦ Others No Uniform Method to Check Against Results ◆ Good? ◆ Bad? ◆ Reliable?

Cost \$30M & Up, Based on Unfounded Analysis



Types of Interchanges

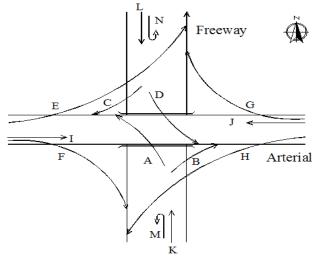




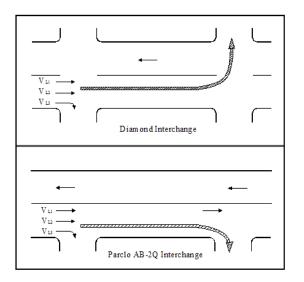
Types of Interchanges (cont'd)

Unique Operational Characteristics of Interchanges

EXHIBIT 26-4. ILLUSTRATION AND NOTATION OF ORIGIN-DESTINATION DEMANDS AT AN INTERCHANGE



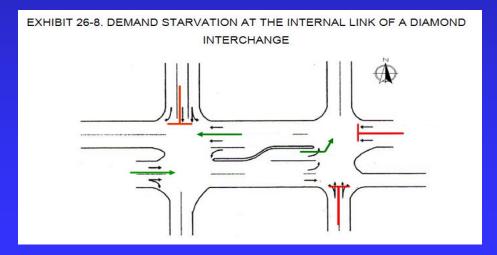




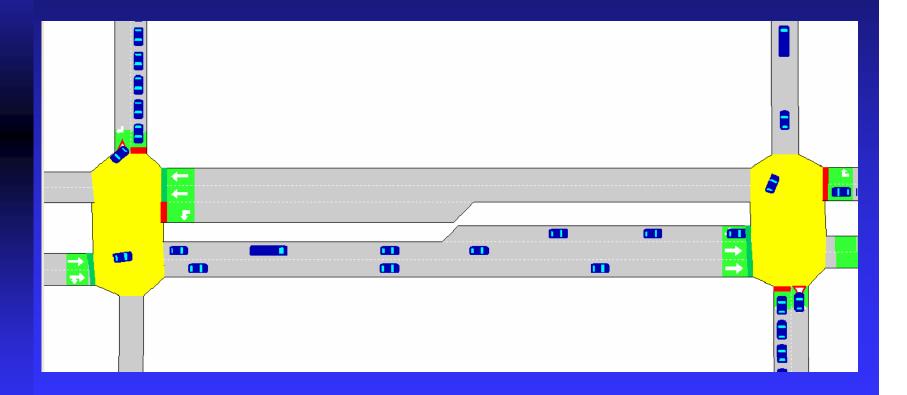


Unique Operational Characteristics of Interchanges (cont'd)

Queue presence in the internal link
Distance between the two intersections.
Demand starvation potential.



Demand Starvation



Level of Service Framework

LOS A B C D E F

Delay <15 15-30 30-55 55-85 85-120 V/C>1.0, Rq>1.0

Performance Measures and LOS

- Average Queue Storage Ratio for each lane group
- V/C for each lane group
- Average Control Delay for each lane group, and for each Origin-Destination
- LOS determination based on Origin-Destinations

Next Steps

HCS+ Now Incorporates Interchanges
 NCHRP 3-60 Extension

- Collecting Additional Field Data
- Incorporate Nearby Intersections
- Include 3-65 Roundabouts
- HCM 2010 to incorporate New Interchange Method

HCS+Interchanges

Software Released HCS+ 5.3 McTrans



